April 26,1994

To: Brent Dammon

Re: Plans for Craig Joseph

From: Bullock Garages, Inc.

1060 N. Perry

Brent Dammon,

Please find here the current plans we have for the building of Craig Joseph's garage. If you have any further questions, please do not hesitate to let me know.

Thank You,

Chad M. Clum

Chad M. Clum
Toledo manager

ask Marc

GENERAL NOTES

1) ALL CONCRETE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3500 PSI
2) ALL CONCRETE TO BE AIR-ENTRAINED
3) PROVIDE DIAGONAL WIND BRACING AS REQUIRED
4) MINIMUM ASSUMED SOIL BEARING CAPACITY = 2000 PSI
5) DESIGN LOAD CALCULATIONS:
   - ROOF:
     LL = 25 PSF
     DL = 10 PSF
     TL = 35 PSF × $\frac{32' \text{ MAXIMUM ROOF SPAN}}{2}$ = 560 PLF
   - WALLS:
     DL = 5 PSF × 8' HEIGHT = 40 PLF
   - FOUNDATION:
     DL = 150 PCF × 2.45 CU. FT. = 369 PLF

     TOTAL LOAD = 969 PLF

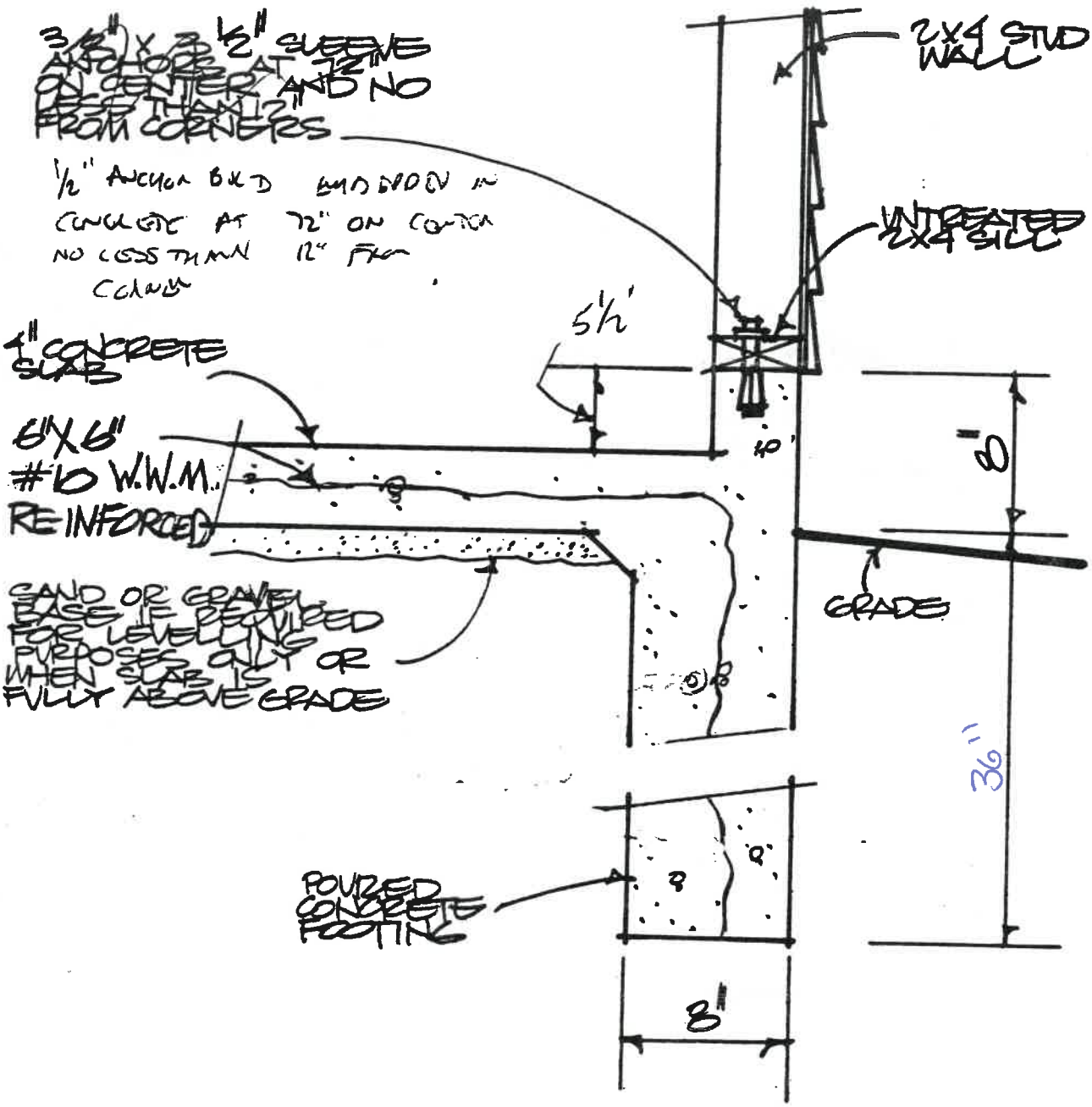
BEARING AREA OF ONE LINEAR FOOT OF 8" WIDE FOUNDATION = 0.67 FT²
969 PLF / 0.67 FT² = 1446.27 PSF
ALLOWABLE SOIL BEARING = 2000 PSF
FACTOR OF SAFETY WITH MAXIMUM LOADING CONDITION = $\frac{2000 \text{ PSF}}{1446 \text{ PSF}}$ = 1.38

SHEET 1	GENERAL NOTES
MAR. 1990	Joseph 92-24883

BULLOCK GARAGES

P.O. BOX 1925
SPRINGFIELD, OHIO

STATE OF OHIO
RICHARD L. TAYLOR JR. 8889
REGISTERED ARCHITECT



SHEET 2A	FOOTING DETAIL
MAR. 1990	SCALE= 1½"=1'.0"

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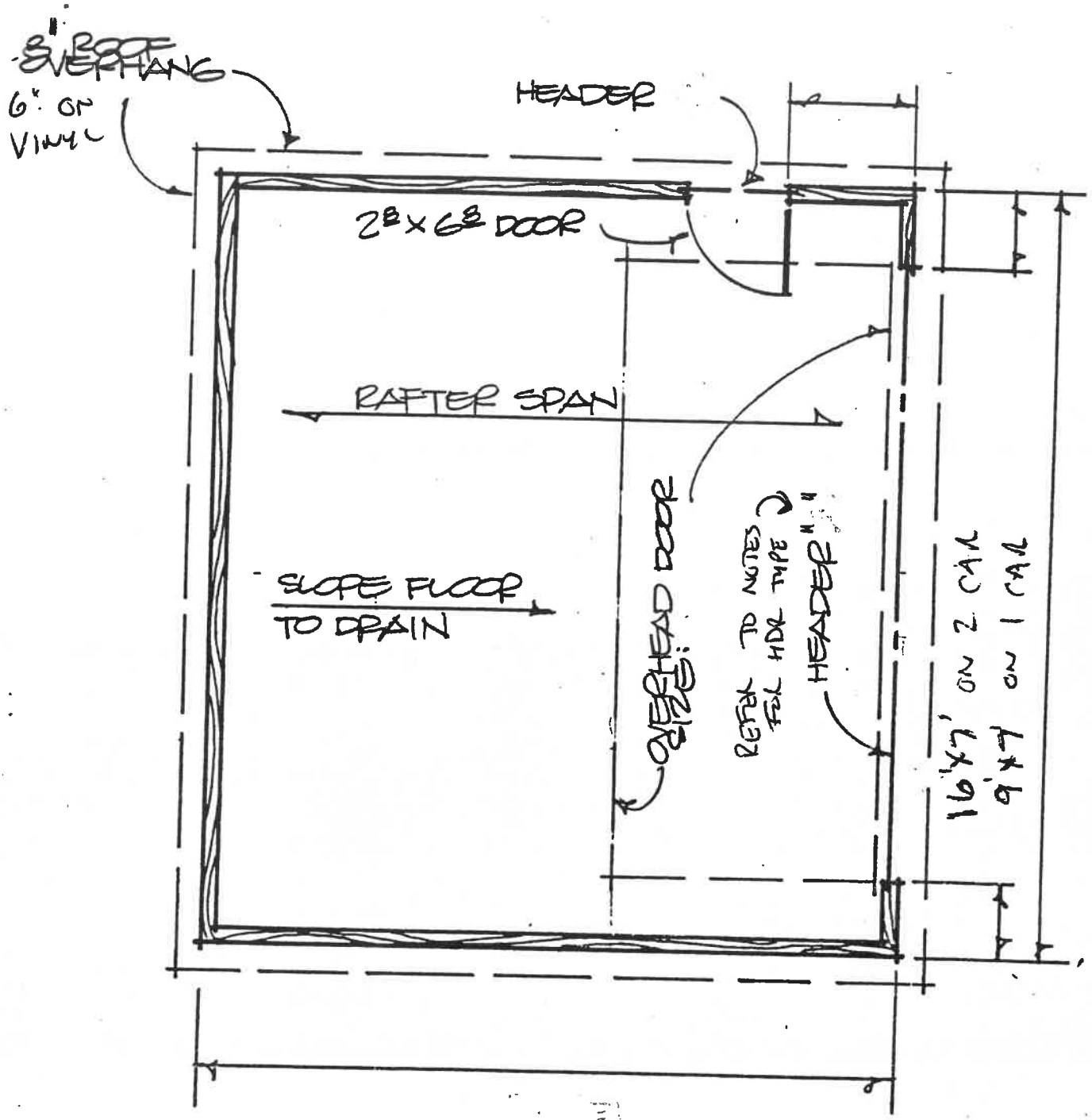

NOT TO SCALE, MAY NOT BE EXACT DOOR LOCATIONS

SHEET 4B	PLAN- REVERSE GABLE
MAR. 1990	SCALE: 1/4" = 1'-0"

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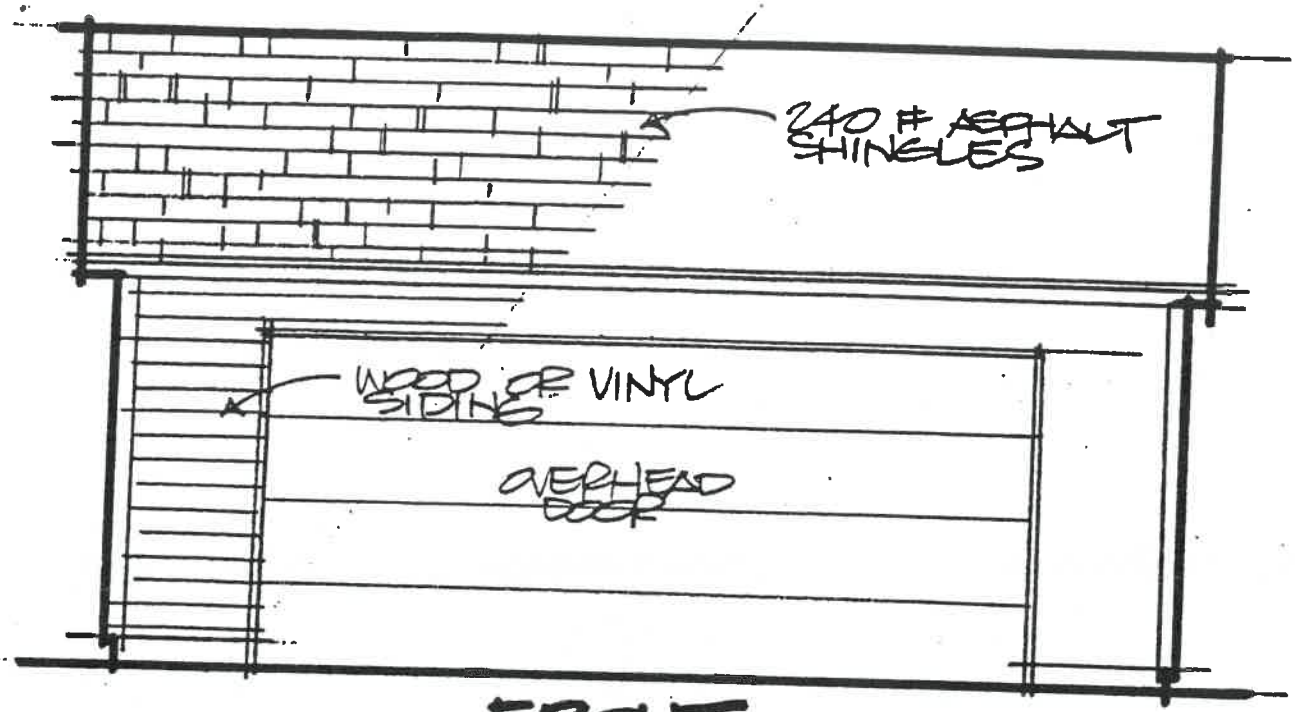
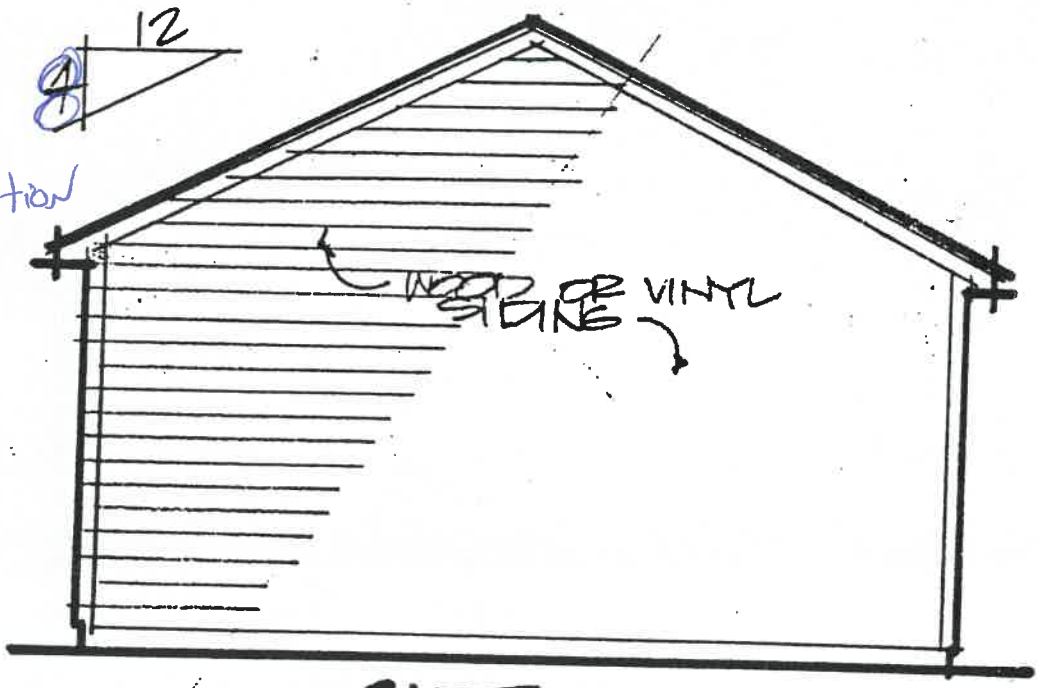


FRONT



Please See Cross Section Following.

SIDE

SHEET 5B	ELEVATIONS - REVERSE GABLE
MAR. 1990	SCALE: 1/4" = 1'-0"

BULLOCK GARAGES

P.O. BOX 1925
SPRINGFIELD, OHIO

STATE OF OHIO
RICHARD L. TAYLOR JR. 8889
REGISTERED ARCHITECT

Job	Truss	Qty	Ply		99721 AD#1
155	24A8	1	1	4/12	

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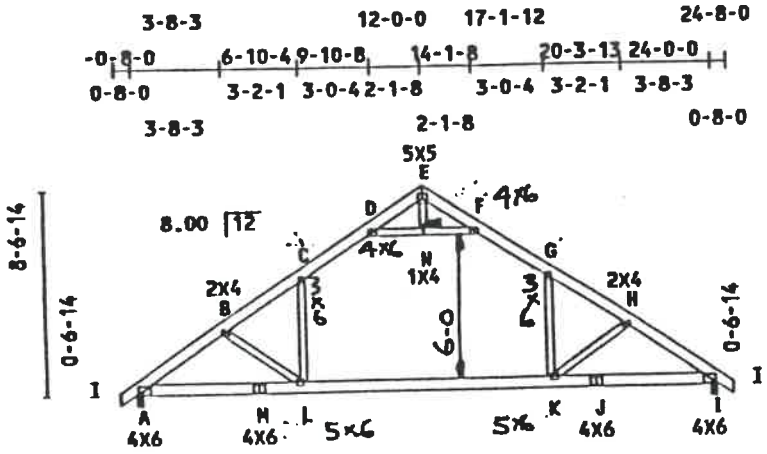
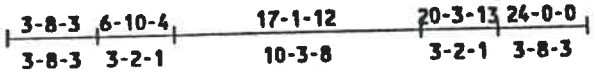


24" O.C.



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	(in)	(l)	l/defl	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.94	Vert(LL)	0.37	L	772	M20(20ga)	199/146
TCDL	7.0	Lumber Increase	1.15	BC	0.78	Vert(TL)	0.41	L	695		
BCLL	0.0	Rep Stress Incr	YES	WB	0.34	Horz(TL)	0.04	I	n/a		
BCDL	10.0	Code	TPI			Min Length / LL defl = 360				(Matrix)	

LUMBER

TOP CHORD	2 X 6 SPF No.2
BOT CHORD	2 X 6 SPF No.2
WEBS	2 X 4 SPF Stud
OTHERS	2 X 4 SPF Stud

BRACING

TOP CHORD	Sheathed
BOT CHORD	Sheathed
JOINT(S)	N

REACTIONS (lbs/size) A=1415/0-3-8, I=1415/0-3-8
Max Horz A=-614(load case 2)

FORCES

TOP CHORD	A-B=-2246, B-C=-1932, C-D=-1395, D-E=72, E-F=72, F-G=-1395, G-H=-1932, H-I=-2246
BOT CHORD	I-J=1801, J-K=1801, K-L=1393, L-M=1801, A-M=1801
WEBS	D-N=-1603, F-N=-1603, B-L=-507, C-L=723, E-N=341, G-K=723, H-K=-507

NOTES

1) This truss has been designed for the wind loads generated by 70.0 m.p.h. winds at 25.0 feet above ground level, using 7.0 p.s.f. top chord dead load and 10.0 p.s.f bottom chord dead load, 101.0 miles from hurricane oceanline, on a category I enclosed building, of dimensions 50.0 by 100.0 with exposure C (ASCE 7-88).

LOAD CASE(S)

1) Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lbs per ft)
Vert:A-B=-74.0, B-C=-74.0, C-D=-74.0, D-E=-74.0, E-F=-74.0, F-G=-74.0, G-H=-74.0, H-I=-74.0, I-J=-20.0, J-K=-20.0, K-L=-60.0, L-M=-20.0, A-M=-20.0, D-N=-20.0, F-N=-20.0
Concentrated Loads (lbs)
Vert:A=-49.3, I=-49.3

PAUL W. LEBARRON REGISTERED 10449 STATE OF WEST VIRGINIA PROFESSIONAL ENGINEER

Paul W. LeBarron

STATE OF KENTUCKY JAMES N. BONDOR 14081 REGISTERED PROFESSIONAL ENGINEER

GENERAL NOTES

1) ALL CONCRETE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3500 PSI

2) ALL CONCRETE TO BE AIR-ENTRAINED

3) PROVIDE DIAGONAL WIND BRACING AS REQUIRED

4) MINIMUM ASSUMED SOIL BEARING CAPACITY = 2000 PSF

5) DESIGN LIVE LOADS:

ROOF = 25 PSF

6) A. BOX BEAM HEADER ON NON LOAD BEARING WALL

B. DBL 2" X 12" WITH 1/2" Plywood BETWEEN FOR 9'X7' OPNG. ON LOAD BEARING WALL

C. LAMINATED 12" X 3 7/16" FOR 10' OPENING AND LARGER ON LOAD BEARING WALL

SHEET 1	GENERAL NOTES
JAN. 1991	

BULLOCK GARAGES

P.O. BOX 1925
SPRINGFIELD, OHIO

STATE OF OHIO
RICHARD L TAYLOR JR. 8889
REGISTERED ARCHITECT

Job	Truss	Truss Type	Qty	Ply	
159	22AS	ATTIC	1	1	BULLOCK

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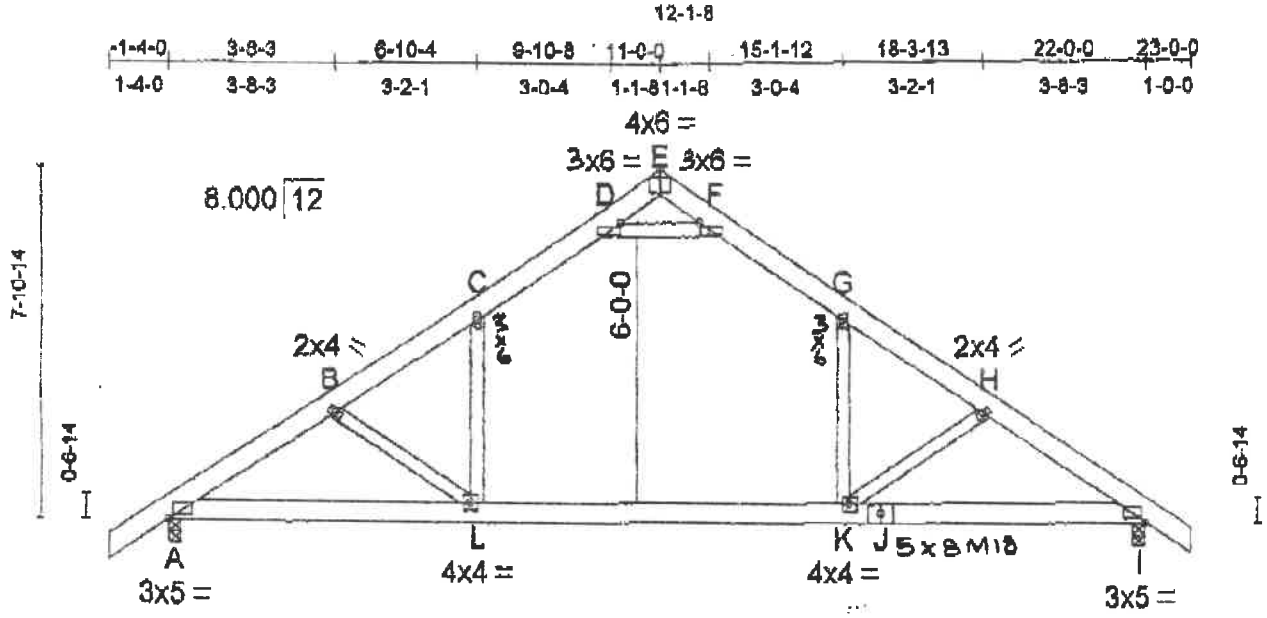

3-8-3	6-10-4	15-1-12	18-3-13	22-0-0
3-8-3	3-2-1	8-3-8	3-2-1	3-8-3

Plate Offsets (X,Y): [A:1.0,1.4], [E:0.0,5.1], [D:0.3,3.1], [F:0.3,3.1], [I:1.0,1.4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	(in)	(loc)	l/defl	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.92	Vert(LL)	0.27	L/K	966	M20(20ga)	199/146
TCDL	10.0	Lumber Increase	1.15	BC	0.74	Vert(TL)	0.45	L/K	580		
BCLL	0.0	Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.04	I	n/a		
BCDL	10.0	Code	TPI	(Matrix)		Min Length / LL defl = 360				Weight: 91 (lbs)	

LUMBER
TOP CHORD 2 X 6 SPF No.2
BOT CHORD 2 X 6 SPF No.2
WEBS 2 X 4 SPF No.3

BRACING
TOP CHORD Sheathed
BOT CHORD Rigid ceiling directly applied, or 10-00-00 on center bracing.

REACTIONS (lbs/size) A=1482/0-3-8, I=1456/0-3-8
Max Horz A=221 (load case 3)

FORCES
TOP CHORD A-B=-2235, B-C=-1901, C-D=-1339, D-E=952, E-F=952, F-G=-1339, G-H=-1901, H-I=-2235
BOT CHORD A-J=1788, J-K=1788, K-L=1360, A-L=1788
WEBS D-F=-2604, B-L=-532, C-L=749, G-K=749, H-K=-532

NOTES
1) This truss has been checked for unbalanced loading conditions about joint E.
2) This truss has been designed for the wind loads generated by 80.0 m.p.h. winds at 25.0 feet above ground level, using 5.0 p.s.f. top chord dead load and 2.0 p.s.f bottom chord dead load, 100.0 miles from hurricane oceanline, on a category I enclosed building, of dimensions 45.0 by 27.0 with exposure C (SBCCI 1205.1-1988). Lumber increase = 1.33, Plate increase = 1.33. No end verticals are exposed.
3) Ceiling load (5.0 psf) on member(s). D-F
4) Bottom chord live load (30.0 psf) applied only to room. L-K

LOAD CASE(S) Standard